

#### **DISCLAIMER**

This document provides practicing engineers and building officials with a resource document for understanding the behavior of steel moment-frame buildings in earthquakes. It is one of the set of six State of the Art Reports containing detailed derivations and explanations of the basis for the design and evaluation recommendations prepared by the SAC Joint Venture. The recommendations and state of the art reports, developed by practicing engineers and researchers, are based on professional judgment and experience and supported by a large program of laboratory, field, and analytical research. No warranty is offered with regard to the recommendations contained herein, by the Federal Emergency Management Agency, the SAC Joint Venture, the individual joint venture partners, or the partner's directors, members or employees. These organizations and their employees do not assume any legal liability or responsibility for the accuracy, completeness, or usefulness of any of the information, products or processes included in this publication. The reader is cautioned to review carefully the material presented herein and exercise independent judgment as to its suitability for application to specific engineering projects. This publication has been prepared by the SAC Joint Venture with funding provided by the Federal Emergency Management Agency, under contract number EMW-95-C-4770.

**Cover Art.** The beam-column connection assembly shown on the cover depicts the standard detailing used in welded steel moment-frame construction prior to the 1994 Northridge earthquake. This connection detail was routinely specified by designers in the period 1970-1994 and was prescribed by the *Uniform Building Code* for seismic applications during the period 1985-1994. It is no longer considered to be an acceptable design for seismic applications. Following the Northridge earthquake, it was discovered that many of these beam-column connections had experienced brittle fractures at the joints between the beam flanges and column flanges.

# **State of the Art Report on Performance Prediction and Evaluation of Steel Moment-Frame Buildings**

## **SAC Joint Venture**

A partnership of **Structural Engineers Association of California (SEAOC) Applied Technology Council (ATC)** California Universities for Research in Earthquake Engineering (CUREe)

## Prepared for the SAC Joint Venture Partnership by Douglas A. Foutch

Department of Civil and Environmental Engineering University of Illinois at Urbana-Champaign

## **Project Oversight Committee**

William J. Hall, Chair

Shirin Ader	James R. Harris	Duane K. Miller
John M. Barsom	Richard Holguin	John Theiss
Roger Ferch	Nestor Iwankiw	John H. Wiggins
Theodore V. Galambos	Roy G. Johnston	
John Gross	Len Joseph	

### **SAC Project Management Committee**

SEAOC: William T. Holmes	Program Manager: Stephen A. Mahin
ATC: Christoper Rojahn	Project Director for Topical Investigations:
CUREe: Robin Shepherd	James O. Malley
•	Project Director for Product Development:
	Ronald O. Hamburger

### **Topical Investigation Team**

James Anderson	Gary C. Hart
C. Allin Cornell	Y. K. Wen

Techn	ical Ad	vis	01	y	Pa	ane	ŀ
_							

Vitelmo V. Bertero	Lawrence G. Griffis	Thomas A. Sabol
Bruce Ellingwood	Edwin T. Huston	Tom Schlafly
Theodore V. Galambos	Harry Martin	

#### **SAC Joint Venture**

SEAOC: www.seaoc.org ATC: www.atcouncil.org CUREe: www.curee.org

September 2000

#### THE SAC JOINT VENTURE

SAC is a joint venture of the Structural Engineers Association of California (SEAOC), the Applied Technology Council (ATC), and California Universities for Research in Earthquake Engineering (CUREe), formed specifically to address both immediate and long-term needs related to solving performance problems with welded, steel moment-frame connections discovered following the 1994 Northridge earthquake. SEAOC is a professional organization composed of more than 3,000 practicing structural engineers in California. The volunteer efforts of SEAOC's members on various technical committees have been instrumental in the development of the earthquake design provisions contained in the Uniform Building Code and the 1997 National Earthquake Hazards Reduction Program (NEHRP) Recommended Provisions for Seismic Regulations for New Buildings and other Structures. ATC is a nonprofit corporation founded to develop structural engineering resources and applications to mitigate the effects of natural and other hazards on the built environment. Since its inception in the early 1970s, ATC has developed the technical basis for the current model national seismic design codes for buildings; the de facto national standard for postearthquake safety evaluation of buildings; nationally applicable guidelines and procedures for the identification, evaluation, and rehabilitation of seismically hazardous buildings; and other widely used procedures and data to improve structural engineering practice. CUREe is a nonprofit organization formed to promote and conduct research and educational activities related to earthquake hazard mitigation. CUREe's eight institutional members are the California Institute of Technology, Stanford University, the University of California at Berkeley, the University of California at Davis, the University of California at Irvine, the University of California at Los Angeles, the University of California at San Diego, and the University of Southern California. These laboratory, library, computer and faculty resources are among the most extensive in the United States. The SAC Joint Venture allows these three organizations to combine their extensive and unique resources, augmented by subcontractor universities and organizations from across the nation, into an integrated team of practitioners and researchers, uniquely qualified to solve problems related to the seismic performance of steel moment-frame buildings.

#### ACKNOWLEDGEMENTS

Funding for Phases I and II of the SAC Steel Program to Reduce the Earthquake Hazards of Steel Moment-Frame Structures was principally provided by the Federal Emergency Management Agency, with ten percent of the Phase I program funded by the State of California, Office of Emergency Services. Substantial additional support, in the form of donated materials, services, and data has been provided by a number of individual consulting engineers, inspectors, researchers, fabricators, materials suppliers and industry groups. Special efforts have been made to maintain a liaison with the engineering profession, researchers, the steel industry, fabricators, code-writing organizations and model code groups, building officials, insurance and risk-management groups, and federal and state agencies active in earthquake hazard mitigation efforts. SAC wishes to acknowledge the support and participation of each of the above groups, organizations and individuals. In particular, we wish to acknowledge the contributions provided by the American Institute of Steel Construction, the Lincoln Electric Company, the National Institute of Standards and Technology, the National Science Foundation, and the Structural Shape Producers Council. SAC also takes this opportunity to acknowledge the efforts of the project participants – the managers, investigators, writers, and editorial and production staff – whose work has contributed to the development of these documents. Finally, SAC extends special acknowledgement to Mr. Michael Mahoney, FEMA Project Officer, and Dr. Robert Hanson, FEMA Technical Advisor, for their continued support and contribution to the success of this effort.

## **TABLE OF CONTENTS**

LI	ST OF	F FIGURES	ix
LI	ST OF	TABLES	XV
1.	INTI	RODUCTION	1-1
	1.1	Purpose	
	1.2	Background	
	1.3	Performance Prediction and Evaluation for Buildings under Seismic Loads	
	1.4	Critical Issues for Performance Prediction and Evaluation	
	1.5	Objectives	
	1.6	Summary	
2.	PER	FORMANCE-BASED DESIGN AND EVALUATION	2-1
	2.1	Background and Description of Performance-Based Design	
	2.2	Basic Definitions.	
	2.3	Performance Levels	
	2.4	Reliability and Performance-Based Evaluation	
3.	SEIS	SMIC HAZARD AND CODE PROVISIONS	3-1
٠.	3.1	Background and Special SAC Ground Motions	
	J.1	3.1.1 Maximum Considered Earthquake Ground Motions	
		3.1.2 Special Ground Motion Time Histories Developed for the SAC Studies	
	3.2	1997 NEHRP Requirements (BSSC, 1997a) (FEMA, 302)	
		3.2.1 Procedures for Determining Maximum Considered Earthquake and Desig	
		Earthquake Ground Motion Accelerations and Response Spectra	
		3.2.1.1 Maximum Considered Earthquake Ground Motions	
		3.2.1.2 General Procedure for Determining Maximum Considered	
		Earthquake and Design Spectral Response Accelerations	3-6
		3.2.2 Seismic Design Category	
		3.2.2.1 Determination of Seismic Design Category	3-12
		3.2.2.2 Site Limitation for Seismic Design Categories E and F	
		3.2.3 Occupancy Importance Factor	
	3.3	Comparison of Seismic Demand for Design from Various Specifications	3-13
4.	ANA	ALYSIS METHODS FOR EVALUATION OF BUILDINGS	4-1
	4.1	Introduction	4-1
		4.1.1 Background	4-1
		4.1.2 Brief Summary of Past Code Provisions	
	4.2	Description of Analysis Methods	4-1
		4.2.1 1997 NEHRP Equivalent Lateral Force Procedure – N97-LSP	
		(FEMA-302)	
		4.2.1.1 Background	4-2
		4.2.1.2 Design Forces	
		4.2.1.3 Drift Determination	4-4

	4.2.2	1997 UBC Static Force Procedure: UBC97-LSP (ICBO, 1997)	4-6
		4.2.2.1 Background	
	4.2.3	FEMA-273 Linear Static Procedure: F273-LSP (ATC, 1997a)	4-6
		4.2.3.1 Background	4-6
		4.2.3.2 Design Forces	4-6
		4.2.3.3 Acceptance Criteria	4-7
		4.2.3.4 Story Drift Calculation	4-8
	4.2.4	1997 NEHRP Modal Analysis Procedure – N97-MAP (BSSC, 1997a)	4-8
		4.2.4.1 Background	4-8
		4.2.4.2 Design Forces	
	4.2.5	1997 NEHRP Dynamic Linear Time History Procedure – N97-LTH	4-12
		4.2.5.1 Background	4-12
		4.2.5.2 Modeling	4-12
		4.2.5.3 Modes	4-12
		4.2.5.4 Modal Properties	4-13
		4.2.5.5 Damping	
		4.2.5.6 Earthquake Accelerograms	4-13
		4.2.5.7 Calculation of Story Drift Demands	4-15
	4.2.6	FEMA-273 Nonlinear Static Procedure – F273-NSP	4-15
		4.2.6.1 Background	
		4.2.6.2 Design Forces	
		4.2.6.3 Acceptance Criteria	4-18
	4.2.7	Capacity Spectrum Procedure – CSP-NSP (ATC-40)	4-19
		4.2.7.1 Background	
		4.2.7.2 Calculation of Floor Displacements	4-19
		4.2.7.3 Acceptance Criteria	
4.3	Mode	eling of New Steel Moment Frames for Performance Prediction	4-24
	4.3.1	Background	4-24
	4.3.2	Linear Elastic Models	4-24
		4.3.2.1 Linear Centerline Models	4-24
		4.3.2.2 Elastic Models with Panel Zones Included	4-25
		4.3.2.3 Nonlinear Centerline Models	4-27
		4.3.2.4 Nonlinear Models with Panel Zones	4-28
	4.3.3	Nonlinear Springs for Beams, Columns, and Panel Zones	4-33
		4.3.3.1 Reduced Beam Section Connection	
		4.3.3.2 Bolted T-Stub, Partially Restrained, Connection	4-35
		4.3.3.3 Local Buckling Behavior in Columns	4-40
	4.3.4		
	4.3.5	Other Modeling Attributes	
4.4	Deter	mination of Bias Factors	4-47
	4.4.1	Background	4-47
		Calculation of Bias Factors	
4.5	Analy	rtical Studies of Post-Northridge Buildings	4-53
		Description of Building Designs	
		Static Pushover Analysis	

		4.5.3 Drift Demands for Typical Post-Northridge SMRF Buildings	4-62
		4.5.4 Axial Force Demand in Column	4-63
		4.5.5 Other Analysis Results	4-67
	4.6	Effects of Modeling, Structural Configuration, and Other Attributes	
		on System Performance	4-68
	4.7	Recent Advances in the Development of Predictive Methods	4-68
		•	
5.	STA	ATISTICAL AND RELIABILITY FRAMEWORK FOR ESTABLISHING	
	PER	RFORMANCE OBJECTIVES	5-1
	5.1	Background	5-1
	5.2	Performance Levels	5-1
	5.3	Load and Resistance Factor Format for Evaluation and Design of	
		Building Systems at Multiple Performance Levels	5-3
	5.4	Performance Objectives	5-3
	5.5	Performance Evaluation Process for New Buildings	5-4
	5.6	Reliability Format Evaluation Procedures	5-6
		5.6.1 Determination of Median Drift Capacity and Resistance Factors	5-6
		5.6.1.1 Connection Test Protocol and Determination of the Median Local	
		Drift Capacity, $\hat{C}$	5-7
		5.6.1.2 Calculation of Global Stability	
		5.6.1.3 Determination of the Resistance Factor, φ	
		5.6.2 Determination of Demand Factors, $\gamma$ and $\gamma_a$	
		5.6.2.1 Determination of γ	
		5.6.2.2 Determination of $\gamma_a$	
		5.6.3 Determination of $\beta_{UT}$	
		5.6.4 Calculation of the Confidence Factor, $\lambda_{con}$	
	5.7	Modeling of Uncertainty and Randomness in Evaluation Process	
	5.7	5.7.1 Background	
		5.7.2 Buildings Used for the Study	
		5.7.3 Local Variation of the Slope of the Hazard Curve, k	
		5.7.4 Determination of b Value	
		5.7.5 Variabilities in Damping of Structures	
		5.7.6 Variabilities in Orientation of the Ground Motions	
		5.7.7 Uncertainties in Analysis Methods	
		5.7.8 Other Uncertainties	
		5.7.9 Coupling and Double Counting of Uncertainties in Capacity and Demand	
	5.8	Implication for Evaluation of Existing Buildings	
	5.9	Evaluating the Relative Effect of Reducing the Uncertainty in Various Design	
		Parameters from a Safety and Reliability Point of View	5-43
6.	PER	RFORMANCE EVALUATION FOR NEW BUILDINGS	6-1
	6.1	Introduction	6-1
	6.2	Performance Levels	6-1
	6.3	Seismic Hazard and Design Spectra	6-2
		6.3.1 Design Spectral Accelerations for Linear Static Procedures	

		6.3.2 Earthquake Accelerograms for Time History Analysis	6-2
		6.3.3 Concurrence of Seismic Ground Motions	6-2
	6.4	Performance Evaluation	6-3
		6.4.1 Performance Evaluation Process for New Buildings	6-3
		6.4.2 Modeling and Analysis	
		6.4.3 Example for Performance Evaluation of 3-Story Post-Northridge Building	6-5
7	DED	FORMANCE OF ORDINARY AND PARTIALLY RESTRAINED	
•		EL MOMENT FRAMES	7 1
	7.1	Background	
	7.1	Effects of Panel Zone Strength and Stiffness on Member and Frame Deformation	
	1.2	Demands	
	7.3		
	1.3	Effects of Weak-Column Designs	
		7.3.1 Background	
		7.3.2 Features of Weak-Column Designs Used for the Study	
		7.3.3 Evaluation of Response of WCSB Buildings	
		7.3.4 Performance Evaluation of WCSB Buildings	
	<b>7</b> 4	7.3.5 Summary of Results for WCSB Buildings	
	7.4	Ordinary Moment Frame Buildings with Partially Restrained Connections	
		7.4.1 Background	
		7.4.2 Stiffness for PR Connection	
		7.4.3 Evaluation of Buildings with T-stub PR Connections	
		7.4.3.1 Background	
		7.4.3.2 Description of Buildings Investigated	
		7.4.3.3 Modeling T-Stub Connections	
		7.4.3.4 Response of the Buildings with PR Connections	
		7.4.4 Performance Evaluation of Buildings with PR Connections	
		7.4.5 Summary of Results for the Buildings with T-Stub PR Connections	
	7.5	Evaluation of Buildings with End-Plate Connections	
		7.5.1 Background	7-37
		7.5.2 Summary of Findings for End-Plate Connection from Connection	
		Performance Team	
		7.5.3 Summary for the Response of the Buildings with End-Plate Connections	7-46
	7.6	Evaluation of Buildings with Clip Angle PR Connections	7-47
		7.6.1 Background	7-47
		7.6.2 Summary of Findings for Clip Angle Connections from Connection	
		Performance Team	7-47
		7.6.3 Properties of Clip Angle Connections for this Study	7-50
		7.6.4 Description of Building Investigated	7-53
		7.6.5 Response of Buildings with Clip Angle PR Connections	
		7.6.5.1 Demand Responses	7-55
		7.6.6 Performance Evaluation of Buildings with Clip Angle Connections	
		7.6.7 Summary of Results for Buildings with Clip Angle Connections	

	7.7	Summary of Results and Conclusions for Seismic Behavior of Frames with PR Connections	7-60
8.		FORMANCE EVALUATION OF EXISTING BUILDINGS	
	8.1	Introduction	
	8.2	Evaluation and Rehabilitation Objectives and Process	
		8.2.1 Evaluation and Rehabilitation Objectives	
		8.2.2 Evaluation Process	
	8.3	General Requirements	
		8.3.1 Scope	
		8.3.2 Performance Levels and Objectives	
		8.3.3 Seismic Hazard and Design Spectrum	
	8.4	Material Properties and Condition Assessment	
		8.4.1 General	
		8.4.2 Properties of In-Place Materials and Components	
		8.4.2.1 Material Properties	
		8.4.2.2 Component Properties	
		8.4.3 Condition Assessment	
		8.4.3.1 General	8-4
		8.4.3.2 Scope and Procedures	
		8.4.3.3 Quantifying Results	8-5
	8.5	Analytical Evaluation Methods for Existing Steel Moment Frames	
		8.5.1 General	
		8.5.1.1 Design Forces Using Various Analysis Procedures	8-6
		8.5.2 Modeling and Analysis	8-6
		8.5.2.1 General	8-6
		8.5.2.2 Stiffness for Analysis	8-7
		8.5.2.3 Modeling Nonlinear Behavior of Connections	8-7
	8.6	Performance Evaluation of Buildings Designed and Constructed Before the	
		Northridge Earthquake	8-10
		8.6.1 Background	8-10
		8.6.2 Expected Performance of Existing Welded Steel Moment Frames Based	
		on Year of Construction	8-14
		8.6.3 Expected Performance of Existing Buildings Based on Stiffness	8-21
		8.6.4 Performance Prediction using Confidence Level Calculations	8-24
9.	PER	FORMANCE EVALUATION OF DAMAGED BUILDINGS	9-1
	9.1	Introduction	9-1
	9.2	Performance Levels and Objectives.	9-2
	9.3	Seismic Hazard and Design Spectrum	9-2
	9.4	Material Properties and Condition Assessment	9-2
	9.5	Modeling and Analysis of Damaged Buildings	9-3
		9.5.1 General	9-3
		9.5.2 Modeling and Analysis of FR and PR Steel Moment Frames	9-3

9.6	Analytical Methods for Performance Prediction and Evaluation of Damaged	
	Buildings	9-3
	9.6.1 Background	9-3
9.7	Performance Prediction using Confidence Level Calculations	
9.8	Summary	
APPEN	DIX A. EXAMPLE FOR PERFORMANCE EVALUATION PROCEDURE	A-1
	Performance Evaluation Procedure	
	Example for Performance Evaluation for 9-Story Buildings	
11.2	A.2.1 LA 9-Story Post-Northridge Building Using RBS Connections	
	A.2.2 LA 9-Story Pre-Northridge Building Using Brittle Connections	
	A.2.3 LA 9-Story Pre-Northridge Damaged Building After 2/50 Accelerogram	
	A.2.4 LA 9-Story Pre-Northridge Damaged Building After 50/50 Accelerogr	
Δ3	Summary	
11.5	Summary	
APPEN	DIX B. PERFORMANCE EVALUATION COEFFICIENTS AND BIAS FAC	TORS
711 1 1211.	FOR NEW AND EXISTING BUILDINGS	
R 1	Performance Evaluation Coefficients	
B.2	Bias Factors	
<b>D</b> .2	B.2.1 Post-Northridge Bias Factors	
	B.2.2 Pre-Northridge Bias Factors	
	D.2.2 The Problem age Blus I detois	13
REFERI	ENCES, FEMA REPORTS, SAC REPORTS, ACRONYMS,	
	IST OF SYMBOLS	R-1
		1
SAC PE	IASE II PROJECT PARTICIPANTS	S-1
~		· · · · · · · · · · · · · · · · · · ·