



**Background  
Document**

**Local and Lateral-Torsional Buckling of  
Wide-Flange Beams**

**Report No. SAC/BD-99/20**

**SAC Joint Venture**

A partnership of

**Structural Engineers Association of California (SEAOC)**

**Applied Technology Council (ATC)**

**California Universities for Research in Earthquake Engineering (CUREe)**

By

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Submitted for distribution to

**SAC Joint Venture**

**650-595-1542**

**<http://www.sacsteel.org>**

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## DISCLAIMER

This document is one of a series documenting background information related to Phase II of the FEMA-funded SAC Steel Project. It is being disseminated in the public interest to increase awareness of the many factors which contribute to the seismic performance of steel moment frame structures. The information contained herein is not for design use and is not acceptable to specific building projects. This report has not been reviewed for accuracy, and the SAC Joint Venture has not verified any of the results presented. **No warranty is offered with regard to the recommendations contained herein, by the Federal Emergency Management Agency, the SAC Joint Venture, the individual joint venture partners, or the partner's directors, members or employees. These organizations and their employees do not assume any legal liability or responsibility for the accuracy, completeness, or usefulness of any of the information, products or processes included in this publication. The reader is cautioned to review carefully the material presented herein and exercise independent judgment as to its suitability for application to specific engineering projects.** This publication has been prepared by the SAC Joint Venture with funding provided by the Federal Emergency Management Agency, under contract number EMW-95-C-4770.



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## THE SAC JOINT VENTURE

SAC is a joint venture of the Structural Engineers Association of California (SEAOC), the Applied Technology Council (ATC), and California Universities for Research in Earthquake Engineering (CUREe), formed specifically to address both immediate and long-term needs related to solving performance problems with welded, steel moment-frame connections discovered following the 1994 Northridge earthquake. SEAOC is a professional organization composed of more than 3,000 practicing structural engineers in California. The volunteer efforts of SEAOC's members on various technical committees have been instrumental in the development of the earthquake design provisions contained in the *Uniform Building Code* and the *1997 National Earthquake Hazards Reduction Program (NEHRP) Recommended Provisions for Seismic Regulations for New Buildings and other Structures*. ATC is a nonprofit corporation founded to develop structural engineering resources and applications to mitigate the effects of natural and other hazards on the built environment. Since its inception in the early 1970s, ATC has developed the technical basis for the current model national seismic design codes for buildings; the *de facto* national standard for postearthquake safety evaluation of buildings; nationally applicable guidelines and procedures for the identification, evaluation, and rehabilitation of seismically hazardous buildings; and other widely used procedures and data to improve structural engineering practice. CUREe is a nonprofit organization formed to promote and conduct research and educational activities related to earthquake hazard mitigation. CUREe's eight institutional members are the California Institute of Technology, Stanford University, the University of California at Berkeley, the University of California at Davis, the University of California at Irvine, the University of California at Los Angeles, the University of California at San Diego, and the University of Southern California. These laboratory, library, computer and faculty resources are among the most extensive in the United States. The SAC Joint Venture allows these three organizations to combine their extensive and unique resources, augmented by subcontractor universities and organizations from across the nation, into an integrated team of practitioners and researchers, uniquely qualified to solve problems related to the seismic performance of steel moment-frame buildings.

## ACKNOWLEDGEMENTS

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## PREFACE

The primary objectives of the FEMA/SAC Phase II Steel Project are to develop guidelines for the seismic evaluation, inspection, repair, design and construction of moment resisting steel frame buildings. A diverse collection of technical investigations is supporting this effort, including the identification of basic material properties in rolled steel sections; development of appropriate welding materials, details, and inspection procedures; specification of anticipated seismic demands imposed on connections as a result of structural response to strong ground motions; and large-scale connection testing to calibrate and verify the design procedures that are ultimately proposed. Tying these activities together is a series of detailed finite element analyses of various connection configurations to quantify the influence of material properties, geometry, and detailing on predicted behavior. In addition, a series of studies have been performed to incorporate the results of the various investigations into a performance based seismic engineering format that can become the basis of the SAC guidelines. Cost and risk studies and investigations into the past performance of this class of structures were also performed to gather valuable information used in the development of the guidelines and other documents.

The primary responsibility of the Connection Performance team in the Phase II Steel Project is to develop straightforward and reliable design and analysis tools for seismic moment resisting connections in steel frame structures. This report documents the results of a analytical investigation to evaluate the local and lateral-torsion instability of beams in moment resisting frames, based on the finite element method. This study focused on unreinforced moment connections. These analyses indicated that the web and flange slenderness ratios are moderately important parameters in developing large plastic rotations. The effect of the number of lateral restraints was found to be very small. The effect of beam restraint demonstrated that the rate of peak resistance degradation is substantially slower in beams within frames. The effects found in this study were found to be consistent with a parallel study on reduced beam section (RBS) moment connections. This task was identified as part of Task 7.02 of the SAC Phase II program. The analyses were performed at the University of Michigan.

Numerous individuals helped to develop the scope and content of the project and to review a preliminary version of this report. These individuals included members of the Technical Advisory Panel (TAP) for Connection Performance; selected members of the Joining and Inspection TAP; and several members of the Project Oversight Committee. The contributions of these individuals are greatly appreciated.

## ABSTRACT

A comprehensive investigation of a local and lateral-torsional buckling of wide-flange beams in moment-resisting frames was conducted using finite element models of fully restrained connections in full-span and half-span frame configurations. The scope of the investigation was limited to unreinforced connection designs where the beam flanges are not weakened or strengthened in any way. This study was focused on the effects of beam flange and web slenderness, number and location of lateral supports, and axial deformation restraint on the plastic rotation capacity of the connection.

The results of parametric analyses on models with different slenderness characteristics show that beams compact according to the AISC Code criteria can develop 0.03 radian plastic rotation, i.e. sustain a total drift of 4%. Effect of beam web and flange slenderness on plastic rotation capacity is moderate, while the effect of number and location of lateral supports is very small. Axial restraint of beam deformation, provided by the full-span frame, has a small effect evident only at drifts larger than 5%. However, the rate of post-peak resistance degradation in half-span and full-span models is different, requiring the use of different resistance drop rules to evaluate their ultimate rotation capacity. Thus, if a 20% drop rule is used for half-span models, a 10% drop rule should be used for full-span models. While the responses of unreduced and RBS connections are different, the effects of beam slenderness parameters examined in this study are virtually identical. Therefore, the same slenderness code provisions should apply for both unreduced and RBS connections.

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