

Background Document

Through-Thickness Strength and Ductility of Column Flanges in Moment Connections

Report No. SAC/BD-99/02

SAC Joint Venture

A partnership of
Structural Engineers Association of California (SEAOC)
Applied Technology Council (ATC)
California Universities for Research in Earthquake Engineering (CUREe)

By Robert Dexter

University of Minnesota, Minneapolis, MN

Minerva Melendrez

The ATLSS Center, Lehigh University, Bethlehem, PA

Submitted for distribution to SAC Joint Venture 650-595-1542 http://www.sacsteel.org

DISCLAIMER

This document is one of a series documenting background information related to Phase II of the FEMA-funded SAC Steel Project. It is being disseminated in the public interest to increase awareness of the many factors which contribute to the seismic performance of steel moment frame structures. The information contained herein is not for design use and is not acceptable to specific building projects. This report has not been reviewed for accuracy, and the SAC Joint Venture has not verified any of the results presented. No warranty is offered with regard to the recommendations contained herein, by the Federal Emergency Management Agency, the SAC Joint Venture, the individual joint venture partners, or the partner's directors, members or employees. These organizations and their employees do not assume any legal liability or responsibility for the accuracy, completeness, or usefulness of any of the information, products or processes included in this publication. The reader is cautioned to review carefully the material presented herein and exercise independent judgment as to its suitability for application to specific engineering projects. This publication has been prepared by the SAC Joint Venture with funding provided by the Federal Emergency Management Agency, under contract number EMW-95-C-4770.



Background Document

Through-Thickness Strength and Ductility of Column Flanges in Moment Connections

Report No. SAC/BD-99/02

SAC Joint Venture

A partnership of
Structural Engineers Association of California (SEAOC)
Applied Technology Council (ATC)
California Universities for Research in Earthquake Engineering (CUREe)

By
Robert Dexter
University of Minnesota, Minneapolis, MN
Minerva Melendrez
The ATLSS Center, Lehigh University, Bethlehem, PA

Submitted for distribution to SAC Joint Venture 650-595-1542 http://www.sacsteel.org

THE SAC JOINT VENTURE

SAC is a joint venture of the Structural Engineers Association of California (SEAOC), the Applied Technology Council (ATC), and California Universities for Research in Earthquake Engineering (CUREe), formed specifically to address both immediate and long-term needs related to solving performance problems with welded, steel moment-frame connections discovered following the 1994 Northridge earthquake. SEAOC is a professional organization composed of more than 3,000 practicing structural engineers in California. The volunteer efforts of SEAOC's members on various technical committees have been instrumental in the development of the earthquake design provisions contained in the Uniform Building Code and the 1997 National Earthquake Hazards Reduction Program (NEHRP) Recommended Provisions for Seismic Regulations for New Buildings and other Structures. ATC is a nonprofit corporation founded to develop structural engineering resources and applications to mitigate the effects of natural and other hazards on the built environment. Since its inception in the early 1970s, ATC has developed the technical basis for the current model national seismic design codes for buildings; the de facto national standard for postearthquake safety evaluation of buildings; nationally applicable guidelines and procedures for the identification, evaluation, and rehabilitation of seismically hazardous buildings; and other widely used procedures and data to improve structural engineering practice. CUREe is a nonprofit organization formed to promote and conduct research and educational activities related to earthquake hazard mitigation. CUREe's eight institutional members are the California Institute of Technology, Stanford University, the University of California at Berkeley, the University of California at Davis, the University of California at Irvine, the University of California at Los Angeles, the University of California at San Diego, and the University of Southern California. These laboratory, library, computer and faculty resources are among the most extensive in the United States. The SAC Joint Venture allows these three organizations to combine their extensive and unique resources, augmented by subcontractor universities and organizations from across the nation, into an integrated team of practitioners and researchers, uniquely qualified to solve problems related to the seismic performance of steel moment-frame buildings.

ACKNOWLEDGEMENTS

Funding for Phases I and II of the SAC Steel Program to Reduce the Earthquake Hazards of Steel Moment-Frame Structures was principally provided by the Federal Emergency Management Agency, with ten percent of the Phase I program funded by the State of California, Office of Emergency Services. Substantial additional support, in the form of donated materials, services, and data has been provided by a number of individual consulting engineers, inspectors, researchers, fabricators, materials suppliers and industry groups. Special efforts have been made to maintain a liaison with the engineering profession, researchers, the steel industry, fabricators, code-writing organizations and model code groups, building officials, insurance and risk-management groups, and federal and state agencies active in earthquake hazard mitigation efforts. SAC wishes to acknowledge the support and participation of each of the above groups, organizations and individuals. In particular, we wish to acknowledge the contributions provided by the American Institute of Steel Construction, the Lincoln Electric Company, the National Institute of Standards and Technology, the National Science Foundation, and the Structural Shape Producers Council. SAC also takes this opportunity to acknowledge the efforts of the project participants - the managers, investigators, writers, and editorial and production staff - whose work has contributed to the development of these documents. Finally, SAC extends special acknowledgement to Mr. Michael Mahoney, FEMA Project Officer, and Dr. Robert Hanson, FEMA Technical Advisor, for their continued support and contribution to the success of this effort.

PREFACE

The primary objectives of the FEMA/SAC Phase II Steel Project are to develop guidelines for the seismic evaluation, inspection, repair, design and construction of moment resisting steel frame buildings. A diverse collection of technical investigations is supporting this effort, including the identification of basic material properties in rolled steel sections; development of appropriate welding materials, details, and inspection procedures; specification of anticipated seismic demands imposed on connections as a result of structural response to strong ground motions; and large-scale connection testing to calibrate and verify the design procedures that are ultimately proposed. Tying these activities together is a series of detailed finite element analyses of various connection configurations to quantify the influence of material properties, geometry, and detailing on predicted behavior. In addition, a series of studies have been performed to incorporate the results of the various investigations into a performance based seismic engineering format that can become the basis of the SAC guidelines. Cost and risk studies and investigations into the past performance of this class of structures were also performed to gather valuable information used in the development of the guidelines and other documents.

This report documents part of the work carried out by the Materials and Fracture team of the Phase II FEMA/SAC Steel Project. This team was responsible for characterizing the chemical, stiffness, strength, toughness and other properties that characterize wide flange rolled shape sections that are commonly used in construction of steel moment resisting frames in seismic applications. Over the last ten years, tremendous changes have occurred in the production methods of rolled structural steel shapes. In addition to characterizing a variety of mechanical properties in the longitudinal (or rolling) direction, considerable efforts were made to understand the properties of steel in the through-thickness direction.

The project described in this report focuses on the properties of rolled column sections in the through-thickness direction. Welded test specimens, representing some of the restraint and residual stress conditions present in column flanges in typical beam to column connections, were designed, fabricated and tested. Because the focus of this study was on the ultimate behavior of the column flange, weld and pull plate material was made substantially stronger than the column base material. A variety of member sizes, strain rates, boundary conditions, weld details and properties were studied. This report comprises part of the work completed as part of Task 5.1.2 of the FEMA/SAC Phase II Program.

Numerous individuals helped to develop the scope and content of the project and to review a preliminary version of this report. These individuals included members of the Technical Advisory Panel (TAP) for Materials and Fracture, selected members of the Joining and Inspection TAP; the Project Management Committee, and several members of the Project Oversight Committee. The contributions of these individuals are greatly appreciated.

FORWARD

The tests described in this report were conducted at Lehigh University primarily by Minerva Melendrez under the direction of Robert Dexter. In August 1997, Robert moved from Lehigh University to the University of Minnesota. Robert Dexter continued to direct the project, although the remainder of the testing was still conducted at Lehigh University.

ABSTRACT

More than forty tee joints were fabricated with high-strength (690 MPa yield strength) "pull" plates welded transversely to opposite flanges of short 610 mm lengths of heavy Grade 50 and Grade 65 column sections. The tee-joint specimens were tested in tension through the pull plates. The tests were performed to determine strength, deformation, and fracture behavior of the flanges of wide-flange column sections when loaded in the through-thickness direction under constrained conditions similar those of a welded beam-to-column the connection. (Each pull plate represents a beam tension flange.) The through-thickness strength of the column flanges exceeded 690 MPa in these tests. This result can be explained by the existence of triaxial constraint of the column flange material, which creates hydrostatic tension stresses, raising the apparent through-thickness strength. This effect is an inherent consequence of the Von-Mises and other yield criteria. Three-dimensional finite-element analyses of these specimens using the Von-Mises yield criterion predict this effect and give results consistent with the experiments.

Table of Contents

For	reword	ii
Abs	stract	ii
Exe	ecutive Summary	v
1.0	INTRODUCTION	1
1.1	Background	1
	1.1.1 Through-thickness failure modes and performance requirements	1
	1.1.2 The effect of weld reinforcement	4
	1.1.3 Maximum beam flange force levels	5
	1.1.4 Northridge Earthquake fractures and their significance with	
	respect to through-thickness properties of column flanges	7
	1.1.5 Typical results of uniaxial through-thickness tensile tests	9
	1.1.6 The effect of constraint in the expected through-thickness behavior	10
	1.1.7 The effects of residual and applied stresses in the	
	longitudinal and transverse directions	12
	1.1.8 The effects of continuity plates	13
	1.1.9 The failure mode	14
1.2	Objectives	15
1.3	Scope of the Testing	15
2.0	DESCRIPTION AND RESULTS OF EXPERIMENTS	16
	Description of tee-joint test specimen	16
	2.1.1 Groove welds joining the pull plates to the column section	20
	2.1.2 Weld reinforcement	22
	2.1.3 Continuity plate details	23
2.2	Steel grades and column shapes selected for testing	24
	Material characterization tests	26
	2.3.1 Chemical analysis	26
	2.3.2 Charpy V-Notch tests	29
	2.3.3 Weld macrosections and hardness traverse	31
	2.3.4 Tensile tests	33
	2.3.4.1 Northwestern Steel and Wire	34
	2.3.4.2 Nucor-Yamato Steel	35
	2.3.4.3 British Steel	36
	2.3.4.4 Trade ARBED	38
	2.3.4.5 Continuity plate, pull-plate, and weld metal	40
	2.3.4.6 Summary of tensile test results	41
2.4	Tee-joint tests	42
	2.4.1 Test matrices	42
	2.4.2 Test setup and procedure	46
	2.4.3 Tee-joint test results	48
	2.4.3.1 The 102 mm wide pull-plate test loaded dynamically	51

2.4.3.2 The 102 mm wide pull-plate test loaded quasi-statically	54
2.4.3.3 The 305 mm wide pull-plate tests	55
2.4.3.4 The 152-mm wide and notched pull-plate tests	60
2.4.3.5 Results from specimens with details designed to induce brittle fracture	61
2.4.3.6 Results from specimen with no continuity plates	
(only case with actual through-thickness failure)	63
2.4.3.7 Results from specimens with details designed to induce bending and prying	65
2.4.3.7 Summary and Discussion	67
3.0 ANALYSIS	68
3.1 Finite element model	69
3.2 Comparison to stress distribution observed in tee-joint tests	75
3.2.1 102 mm wide pull-plate test (quasi-static strain rate)	75
3.2.2 305 mm wide pull-plate test (quasi-static strain rate)	76
3.2.3 Variation of thickness and width	77
3.2.4 Summary	80
o.a. i building	
4.0 CONCLUSIONS AND RECOMMENDATIONS	81
	81
	83
	05
5.0 REFERENCES	84
6.0 ACKNOWLEDGEMENT	86
LISTING OF SAC REPORTS	87